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For angle legs $\geq 5"$, the potential for two rows of bolts exists. Thus, the gage

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"g1" is analogous to "g" for the other angle leg, and gage "g2" is the spacing between the first and second row of bolts. (See illustration and table in AISC 13th Edition Manual page 1-46.)

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Z is the current books (the 13th edition) way of calculating bending stress. So $f_b = M/Z$ now instead of M/S . Also, Z is usually at least 10% greater than S was.

S and Z AISC steel design -

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and the American Iron and Steel Institute (AISI) for cold-formed steel members, such as Z and C roof purlins and wall girts.

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From AISC LRFD Table 7-10, the design shear strength of a 7/8" ϕ A325 bolt with the threads included in the shear plane

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(snugged-tight joint) $\phi FV = 21.6$ kips
[from $\phi F_n A_b$ (Table J3.2, where $\phi = 0.75$,
 $F_n = 48$ ksi, and $A_b = 0.601$ in²)] No. of
bolts required = $116.1 / 21.6 = 5.4$ USE
6 bolts to connect the T-stubs to the
beam flanges

Design of Bolted and Welded Connection per AISC LRFD 3rd ...

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WIDE FLANGE SHAPES - JFE

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design. The material presented in this
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the IBC 2006 ASD is a design method by which the members are sized such that the allowable strength is not less than the Required Strength, therefore to assure structural safety: $R_a \leq R_n/\Omega$ (Equation B3-2, AISC 13 th Edition)

